

April 4, 2023

Addendum 1

Notice to all holders of Bid Documents for **Chamberlain Street Retaining Wall Reconstruction Project – BGS# 3172**

The following Changes are incorporated into the contract documents for Chamberlain Street Retaining Wall Reconstruction Project – BGS# 3172.

**Addendum 1**

1. The geotechnical report noted on the contract drawings is provided on the following sheets.
2. The plan drawings from the previous Chamberlain St Parking Lot Improvements Project, BGS #3172 by SJR Engineering and noted on the contract documents are provided on the following sheets.
3. The boundary survey date 7/14/2022 noted on the contract drawings is provided on the following sheets.
4. Contract Bid Form 00 41 13 (2 pages) is provided on the following sheets. Please insert after page 9 of the Project Bid Book as pages 9a and 9b. In pen and ink change the page numbers for 00 41 13 – CONTRACTOR BID FORM to 9a - 9b and add to the line below the following “00 52 13 SAMPLE CONTRACT AGREEMENT ..... 10-13”
5. In pen and ink, cross out the Bid Tab Table on page 9.
6. Add the following in pen and ink as a new paragraph at the end of page 50. “The quantities provided on the plans and removed from page 9 of the Project Bid Book are for informational purposes only. Payment for work shall be based on the base bid amount in the Contractor Bid Form (Section 00 41 13), and the Contract Agreement Section 00 52 13 Article 1. This shall supersede the method of measurement and the basis of payment for each individual item noted in the MaineDOT Standard Specifications and the project specific Special Provisions included in the bid book. Payment will not be made based upon the provided quantities and associated unit prices.”
7. On the first page of the Project Bid Book – In pen and ink below prepared by, add “March 9<sup>th</sup>, 2023”.

Please take the following question and answers from the pre-bid into account when developing your bid.

1. **Question** - A Contractor asked if the existing raised bed planters can be replaced with new planters as they appear to be readily available kits? **Answer** – As some of the existing planters have deteriorated to a point where it is unlikely that they can be removed and reset it is acceptable to replace the planters with new materials matching/similar to the existing materials and dimensions of the existing.
2. **Question** – A contractor asked if the tree and stump to the east of the tree and stump to be removed and noted as “contractor to protect existing tree (typ.)” on plan sheet 3 as it was felt the root system of the tree would be significantly impacted by construction activities? **Answer** – The additional tree can be removed if contractor’s proposed excavation will significantly impact the root system of the tree.

3. **Question** – A contractor asked if the existing gate in the composite stockade security fence on Chamberlain Street is the only access to the project site? **Answer** – The gate and through the section of the composite stockade security fence to be removed and reset and behind the temporary security fence shall be the contractor’s only access to the back side of the existing and proposed retaining wall.
4. **Question** – A contractor asked if there was a stone wall behind the existing cast in place concrete wall and if so, how would the removal of the wall be paid for? **Answer** – It is unknown if there is a stone wall behind the existing concrete wall as record drawings for the existing wall are not available. Based upon the boring and probes conducted at the site, a stone wall was not encountered during the subsurface exploration program. If there is a change in subsurface conditions during the project, it will be addressed accordingly during construction.
5. **Question** – A contractor asked who installed the existing PVC Stockade Fence? **Answer** - Pine Tree Fencing as noted on Plan Sheet 2, note 27.
6. **Question** – A contractor asked if there is a geotechnical report for the project? **Answer** – The project geotechnical report was provided in Addendum 1.
7. **Question** – The precast supplier asked if a curved block layout could be used instead of the proposed 45 degree block at station 0+39 of the wall? **Answer** – A curved block layout may be used in place of the 45 degree block shown on the plans. The maximum radius at the base of the wall shall be 20 feet.
8. **Question** – The precast supplier asked if top blocks could be substituted for the dual faced blocks called for on plan sheet 4 as the dual faced blocks will require an additional cap block to cover the lifting points for the dual faced block? **Answer** – The dual faced blocks shall be replaced with top blocks. In pen and ink change note 4 on sheet 4 to state “The top course of blocks for all locations shall be top blocks” and the “Dual face block” in the typical wall section to “Top Block”.

Please remember to acknowledge this addendum and date April 4, 2023 in the Contractor Bid Form (Section 00 41 13).



## REPORT

21-0679.2

February 7, 2022

### Explorations and Geotechnical Engineering Services

Retaining Wall Replacement  
Chamberlain Street  
Augusta, Maine

**Prepared For:**

Kleinfelder  
Attention: Keith Wood, P.E.  
16 Commerce Drive, Suite 2  
Augusta, ME 04330

**Prepared By:**

S. W. Cole Engineering, Inc.  
26 Coles Crossing Drive  
Sidney, ME 04330  
T: 207.626.0600

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21-0679.2

February 7, 2022

Kleinfelder  
Attention: Keith Wood, P.E.  
16 Commerce Drive, Suite 2  
Augusta, ME 04330

Subject: Explorations and Geotechnical Engineering Services  
Retaining Wall Replacement  
Chamberlain Street and Higgins Street  
Augusta, Maine

Dear Keith:

In accordance with our Proposal, dated November 16, 2021, we have performed subsurface explorations for the subject project. This report summarizes our findings and geotechnical recommendations, and its contents are subject to the limitations set forth in Appendix A.

## **1.0 INTRODUCTION**

### **1.1 Scope and Purpose**

The purpose of our services was to obtain subsurface information at the site in order to develop geotechnical recommendations relative to foundations and earthwork associated with the proposed construction. Our scope of services included a test boring and auger probe explorations, soils laboratory testing, a geotechnical analysis of the subsurface findings, and preparation of this report.

### **1.2 Site and Proposed Construction**

The site is the existing retaining wall located on southern boundary of the Chamberlain Street and Higgins Street parking area in Augusta, Maine. The existing retaining wall consists of a ±8-to-10-foot tall, cast-in-place (CIP) concrete retaining wall supporting a ±2-foot rise in grade up to a relatively flat lawn area. The retaining wall has an overall length of about 140 feet. We understand the existing CIP wall is failing and will be

replaced. We understand replacement options include replacing the wall in its current location or constructing a new wall in front of the existing wall. We understand the replacement wall type has not been selected however, CIP and Segmental-block retaining walls are to be considered.

Existing site features are shown on the "Exploration Location Plan" attached in Appendix B.

## **2.0 EXPLORATION AND TESTING**

### **2.1 Explorations**

One test boring (B-101) and a series of auger probes were made at the site on January 6, 2022, by S. W. Cole Explorations, LLC. The exploration locations were selected and established in the field by S. W. Cole Engineering, Inc. (S.W.COLE) using measurements from existing site features. The auger probes were drilled at about 6-inch intervals beginning about 1-foot from the wall face to investigate the retaining wall footing geometry. The approximate exploration locations are shown on the "Exploration Location Plan" attached in Appendix B. A log of the test boring and a key to the notes and symbols used on the log are attached in Appendix C.

### **2.2 Field Testing**

The test boring was drilled using hollow-stem augers and the auger probes were drilled using solid-stem augers. The soils in the test boring were sampled at 2-to-5-foot intervals using a split spoon sampler and Standard Penetration Testing (SPT) methods. A Vane Shear Test (VST) was performed where softer cohesive soils were encountered. SPT blow counts and VST results are shown on the logs.

### **2.3 Laboratory Testing**

Soil samples obtained from the explorations were returned to our laboratory for further classification and testing. Atterberg Limits and moisture content test results are noted on the logs. The results of two gradation tests are attached in Appendix D.

### 3.0 SUBSURFACE CONDITIONS

#### **3.1 Soil and Bedrock**

Test Boring: Test boring B-101 was made behind the existing retaining wall and encountered a soils profile generally consisting of topsoil overlying medium dense to dense, silty gravelly sand (fill) to a depth of about 14 feet overlying loose, silty sand with some gravel (fill) to about 20 feet. Below the fill, very loose silty sand with organics (leaf debris) and debris (coal, ash) to a depth of about 21 feet overlying loose, silty sand to about 25 feet overlying medium stiff, silty clay with sandy silt partings to about 36.5 feet overlying granular soils (probable glacial till). The test boring was terminated in the granular soils at a depth of about 38.5 feet.

Refer to the attached boring log for more detailed subsurface information.

Auger Probes: A series of shallow auger probes were made at the toe of wall to interpret the geometry at the existing retaining wall footing.

Exploration No.	Location	Refusal Depth (feet)
P-101	20" from face of wall	4.2
P-102	37" from face of wall	No Refusal
P-103	28" from face of wall	4.3
P-104	33" from face of wall	4.2
P-105	28" from face of wall	4.0
P-106	33" from face of wall	4.1
P-107	34" from face of wall	No Refusal

Based on our auger probe data, we interpret the existing retaining wall footing is located about 4 feet below existing ground surface and extends approximately 2.7 feet from the face of the wall.

#### **3.2 Groundwater**

The soils encountered at the test borings were moist to wet from the ground surface. Saturated soils were encountered at a depth of about 18 feet at the time of exploration. Long term groundwater information is not available. It should be anticipated that groundwater levels will fluctuate, particularly in response to periods of snowmelt and precipitation, as well as changes in site use.

## 4.0 EVALUATION AND RECOMMENDATIONS

### 4.1 General Findings

Based on the subsurface findings, the proposed retaining wall construction appears feasible from a geotechnical standpoint. The principle geotechnical considerations include:

- Undocumented and uncontrolled fills extending to a depth of about 21 feet (interpreted as approximately 6 feet below the wall footing) were encountered behind the existing wall. The fills were generally medium dense to a depth of about 14 feet overlying loose to very loose silty sand with a layer of organics and debris. The fills were underlain by loose silty sands and compressible, medium stiff silty clay.
- Based on the loose to very loose silty sands, presence of organics, and silty clays we recommend the proposed retaining wall consist of a Segmental-block Retaining Wall (SRW) that is more tolerant to potential settlement. Additionally, we recommend the retaining wall be replaced in its current location.
- We recommend the existing CIP retaining wall be removed and the over-excavated area be backfilled with compacted Gravel Borrow up to within 2 feet for finish grades at the toe of wall.
- Following removal of the existing retaining wall, a SRW bearing on properly prepared subgrades appear suitable. We recommend the SRW be embedded at least 1 foot and be founded on a 12-inch layer of Crushed Stone wrapped in a non-woven geotextile fabric underlain by compacted Gravel Borrow. We anticipate the SRW will have reinforcement geotextile for internal stability.
- We recommend the retaining wall be backfilled with a minimum 1-foot (horizontal measure) of Crushed Stone directly behind the wall to reduce hydrostatic pressure. An underdrain pipe with gravity outlet should be installed within the Crushed Stone layer below the retaining wall.
- Subgrades across the site will consist of moisture-sensitive silty sands soils. Earthwork and grading activities should occur during drier, non-freezing weather of Spring, Summer, and Fall.



- Imported Gravel Borrow and Crushed Stone will be needed for construction. The existing fill within the anticipated excavation area are unsuitable for reuse as below the proposed retaining wall or as backfill.

#### **4.2 Site and Subgrade Preparation**

We recommend site preparation begin with the construction of an erosion control system to protect adjacent drainage ways and areas outside the construction limits. Surficial organics, roots and topsoil and the existing retaining wall should be completely removed from the area of proposed construction. As much vegetation as possible should remain outside the construction areas to lessen the potential for erosion and site disturbance.

Following removal of the existing CIP wall, the subgrade soils should be densified prior to backfilling. Soft or pumping soils should be over-excavated. The over-excavated area should be backfilled with compacted Gravel Borrow up to the bottom of the proposed SRW Crushed Stone leveling pad.

In general, subgrades for the proposed retaining wall will consist of silty sand with varying amounts of gravel.

#### **4.3 Excavation and Dewatering**

Excavation work will generally encounter fills consisting of silty sand with varying amounts of gravel. Care must be exercised during construction to limit disturbance of the bearing soils. Earthwork and grading activities should occur during drier, non-freezing weather of Spring, Summer and Fall.

Sumping and pumping dewatering techniques should be adequate to control groundwater in excavations. Controlling the water levels to at least one foot below planned excavation depths will help stabilize subgrades during construction. Excavations must be properly shored or sloped in accordance with OSHA Regulations to prevent sloughing and caving of the sidewalls during construction. Care must be taken to preclude undermining adjacent structures, utilities, and roadways. The design and planning of excavations, excavation support systems, and dewatering is the responsibility of the contractor.

#### **4.4 Retaining Wall**

We recommend the existing CIP retaining wall be replaced with a SRW in its current location to reduce potential for settlement. We understand the proposed wall will be

approximately 150 feet in length with retained a retained height of about 10 feet and tapering down on the western end of the alignment. We recommend the existing retaining wall be completely removed and the over-excavated area backfilled with compacted Gravel Borrow up to about 1 foot from the bottom of the proposed wall.

We recommend the proposed wall be founded on at least 12 inches of compacted Crushed Stone wrapped in a non-woven geotextile fabric, such as Mirafi 160N, bearing on compacted Gravel Borrow. For design of SRW, such as Redi-Rock, we recommend the following geotechnical parameters for design:

<b>Geotechnical Parameters for Segmental Retaining Wall</b>			
<b>Wall Segment</b>	<b>Friction Angle</b>	<b>Cohesion</b>	<b>Unit Weight</b>
Retained Backfill (modified Gravel Borrow)	34 degrees	0 psf	125 pcf
Foundation (Gravel Borrow)	34 degrees	0 psf	125 pcf
Net Allowable Soil Bearing Capacity	1.5 ksf (properly prepared subgrade)		
Anticipated Settlement	1-inch or less (post construction)		
Seismic Site Class (IBC 2015)	E		

\*\* Modified Gravel Borrow consists of MaineDOT Gravel Borrow screened over the 3-inch sieve.

Design of the retaining wall and evaluation of base sliding, overturning, internal stability and global stability of the wall are the responsibility of the wall designer. The wall designer must account for construction loads and future live load conditions.

We recommend the retaining wall be backfilled with a vertical column of Crushed Stone directly behind the face of the wall for drainage. An underdrain should be installed within the Crushed Stone layer below the segmental blocks.

#### **4.5 Backfill and Compaction**

The fill soils should not be re-used below the retaining wall or as backfill. We recommend the following fill and backfill materials in accordance with the 2018 Maine Department of Transportation (MaineDOT) Standard Specification:

Gravel Borrow: Fill to raise grades in over-excavated areas below the retaining wall should meet the requirements of 2020 MaineDOT Standard Specification 703.20 Gravel Borrow.

Modified Gravel Borrow: Backfill for the retaining wall should meet the requirements of 2020 MaineDOT Standard Specification 703.20 Gravel Borrow, except the backfill material shall only contain material passing the 3-inch sieve.

Crushed Stone: Crushed Stone, used for the vertical drain behind the wall, should meet the requirements of 2020 MaineDOT Standard Specification 703.22 Type C Underdrain Aggregate.

Placement and Compaction: Fill and backfill should be placed in horizontal lifts and compacted such that the desired density is achieved throughout the lift thickness with 3 to 5 passes of the compaction equipment. Loose lift thicknesses for grading, fill and backfill activities should not exceed 12 inches. Lift thickness within 3 feet of the retaining wall should be limited to that which can be thoroughly compacted using small, hand operated or walk-behind compaction equipment to avoid over-compaction. We recommend wall backfill materials be compacted to 95 percent of its maximum dry density as determined by ASTM D-1557. We recommend Crushed Stone be compacted with 3 to 5 passes of a vibratory plate compactor having a static weight of at least 500 pounds.

#### **4.6 Weather Considerations**

Construction activity should be limited during wet and freezing weather and the site soils may require drying before construction activities may continue. The contractor should anticipate the need for water to temper fills in order to facilitate compaction during dry weather. If construction takes place during cold weather, subgrades and backfill soils must be protected during freezing conditions. Fill must not be placed on frozen soil; and once placed, the soil must be protected from freezing.

#### **4.7 Design Review and Construction Testing**

S.W.COLE should be retained to review the construction documents to determine that our geotechnical recommendations have been properly interpreted and implemented.

S.W.COLE should also be engaged to observe preparation of bearing surfaces as well as to provide testing of soils during construction to observe compliance with the design concepts, plans, and specifications.

## 5.0 CLOSURE

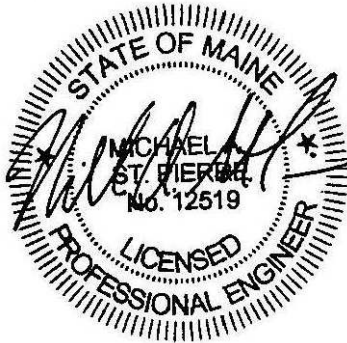
It has been a pleasure to be of assistance to you with this phase of your project. We look forward to working with you during the construction phase of the project.

Sincerely,

**S. W. Cole Engineering, Inc.**

Michael A. St. Pierre, P.E.  
Senior Geotechnical Engineer

MAS:prw



## **APPENDIX A**

### **Limitations**

This report has been prepared for the exclusive use of Kleinfelder for specific application to the proposed Retaining Wall Replacement project on Chamberlain Street and Higgins Street in Augusta, Maine. S. W. Cole Engineering, Inc. (S.W.COLE) has endeavored to conduct our services in accordance with generally accepted soil and foundation engineering practices. No warranty, expressed or implied, is made.

The soil profiles described in the report are intended to convey general trends in subsurface conditions. The boundaries between strata are approximate and are based upon interpretation of exploration data and samples.

The analyses performed during this investigation and recommendations presented in this report are based in part upon the data obtained from subsurface explorations made at the site. Variations in subsurface conditions may occur between explorations and may not become evident until construction. If variations in subsurface conditions become evident after submission of this report, it will be necessary to evaluate their nature and to review the recommendations of this report.

Observations have been made during exploration work to assess site groundwater levels. Fluctuations in water levels will occur due to variations in rainfall, temperature, and other factors.

S.W.COLE's scope of services has not included the investigation, detection, or prevention of any Biological Pollutants at the project site or in any existing or proposed structure at the site. The term "Biological Pollutants" includes, but is not limited to, molds, fungi, spores, bacteria, and viruses, and the byproducts of any such biological organisms.

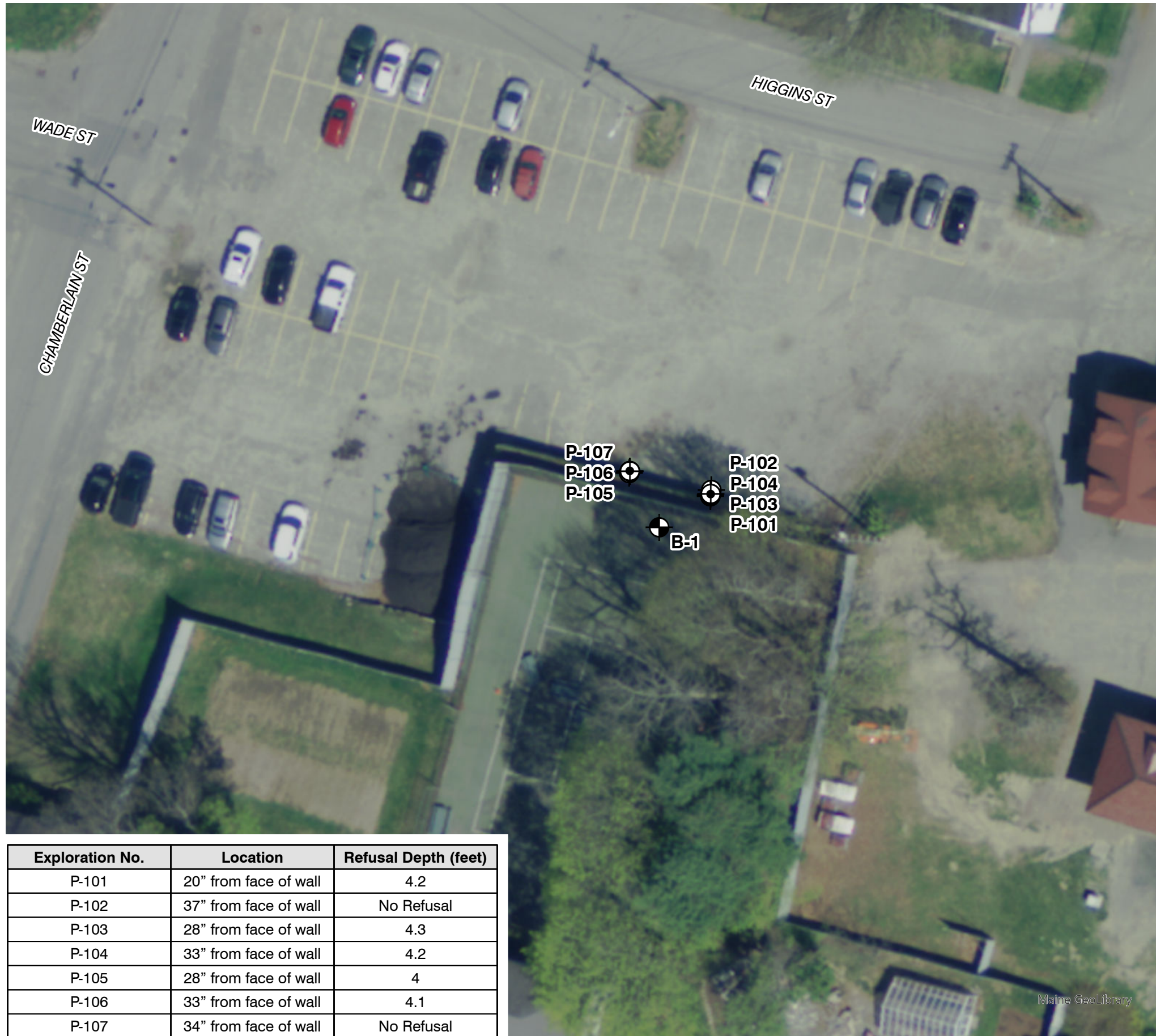
Recommendations contained in this report are based substantially upon information provided by others regarding the proposed project. In the event that any changes are made in the design, nature, or location of the proposed project, S.W.COLE should review such changes as they relate to analyses associated with this report. Recommendations contained in this report shall not be considered valid unless the changes are reviewed by S.W.COLE.

## **APPENDIX B**

### **Figures**





R:\2021\21-0679.2\ArProject\21-0679.2.aprx, 2/7/2022 9:27 AM 21-0679.2 Sheet 1 - ELP, Scale: 1:360, CMORRISON, S. W. COLE ENGINEERING, INC.



Exploration No.	Location	Refusal Depth (feet)
P-101	20" from face of wall	4.2
P-102	37" from face of wall	No Refusal
P-103	28" from face of wall	4.3
P-104	33" from face of wall	4.2
P-105	28" from face of wall	4
P-106	33" from face of wall	4.1
P-107	34" from face of wall	No Refusal

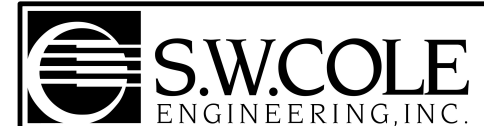


**LEGEND**

-  APPROXIMATE PROBE LOCATION
-  APPROXIMATE BORING LOCATION

**NOTES:**

1. EXPLORATION LOCATION PLAN PREPARED FROM ORTHOIMAGERY ENTITLED "ORTHOREGIONAL2018," PROVIDED BY THE MAINE GEOLIBRARY.
2. THE EXPLORATIONS WERE LOCATED IN THE FIELD BY MEASUREMENTS FROM EXISTING SITE FEATURES.
3. THIS PLAN SHOULD BE USED IN CONJUNCTION WITH THE ASSOCIATED S. W. COLE ENGINEERING, INC. GEOTECHNICAL REPORT.
4. THE PURPOSE OF THIS PLAN IS ONLY TO DEPICT THE LOCATION OF THE EXPLORATIONS IN RELATION TO THE EXISTING CONDITIONS AND PROPOSED CONSTRUCTION AND IS NOT TO BE USED FOR CONSTRUCTION.



KLEINFELDER  
**EXPLORATION LOCATION PLAN**  
 RETAINING WALL REPLACEMENT  
 CHAMBERLAIN STREET  
 AUGUSTA, MAINE

Job No.	21-0679.2	Scale	1" = 30'
Date:	02/07/2022	Sheet	1

## **APPENDIX C**

### **Exploration Logs and Key**





# BORING LOG

**BORING NO.:** B-101  
**SHEET:** 1 of 2  
**PROJECT NO.:** 21-0679.2  
**DATE START:** 1/6/2022  
**DATE FINISH:** 1/6/2022

**CLIENT:** Kleinfelder  
**PROJECT:** Chamberlain Street Retaining Wall  
**LOCATION:** Chamberlain Street, Augusta, Maine

## Drilling Information

**LOCATION:** See Exploration Location Plan    **ELEVATION (FT):** N/A    **TOTAL DEPTH (FT):** 38.6    **LOGGED BY:** Michael St. Pierre  
**DRILLING CO.:** S. W. Cole Explorations, LLC    **DRILLER:** Kevin Hanscom    **DRILLING METHOD:** Hollow Stem Auger  
**RIG TYPE:** Track Mounted Diedrich D-50    **AUGER ID/OD:** 2 1/4 in / 5 5/8 in    **SAMPLER:** Standard Split-Spoon  
**HAMMER TYPE:** Automatic    **HAMMER WEIGHT (lbs):** 140    **CASING ID/OD:** N/A / N/A    **CORE BARREL:** N/A  
**HAMMER EFFICIENCY FACTOR:** 0.91    **HAMMER DROP (inch):** 30  
**WATER LEVEL DEPTHS (ft):** Soils wet below ±14', saturated below ±18'

## GENERAL NOTES:

**KEY TO NOTES AND SYMBOLS:** Water Level  
▽ At time of Drilling    D = Split Spoon Sample    Pen. = Penetration Length    WOR = Weight of Rods    S<sub>v</sub> = Field Vane Shear Strength, kips/sq.ft.  
▽ At Completion of Drilling    U = Thin Walled Tube Sample    Rec. = Recovery Length    WOH = Weight of Hammer    q<sub>u</sub> = Unconfined Compressive Strength, kips/sq.ft.  
▽ After Drilling    R = Rock Core Sample    bpf = Blows per Foot    RQD = Rock Quality Designation    Ø = Friction Angle (Estimated)  
▽ After Drilling    V = Field Vane Shear    mpf = Minute per Foot    PID = Photoionization Detector    N/A = Not Applicable

Elev. (ft)	Depth (ft)	Casing Pen. (bpf)	SAMPLE INFORMATION					Graphic Log	Sample Description & Classification	H <sub>2</sub> O Depth	Remarks
			Sample No.	Type	Depth (ft)	Pen./ Rec. (in)	Blow Count or RQD				
								0.4' Topsoil			
			1D	1-3	24/10	10-13-14-9			Medium dense to dense, brown, moist, Silty Gravelly SAND, (FILL)		
			2D	3-5	24/20	8-11-20-22					
	5		3D	5-7	24/21	24-30-28-35	ID 13733A w = 4.8 %				
			4D	8-10	24/12	21-24-19-17					
	10		5D	10-12	24/8	16-14-15-8					
			6D	12-14	24/0	15-6-11-7			No recovery		
	15		7D	14-16	24/10	3-4-2-2	ID 13734A w = 9.9 %		Loose, brown, wet, Silty SAND, some fine gravel, (FILL)		
			8D	16-18	24/22	2-2-1-1					
	20		9D	20-22	24/20	1-1-1-1			20.0' Very loose, dark brown, wet, Silty SAND with organics (leaf debris) and debris (coal, ash), (Probable Relic Ground Surface / FILL) 20.7' Loose, brown, wet to saturated, Silty SAND with rootlets		

Stratification lines represent approximate boundary between soil types, transitions may be gradual. Water level readings have been made at times and under conditions stated. Fluctuations of groundwater may occur due to other factors than those present at the time measurements were made.

(Continued Next Page)

**BORING NO.:** B-101



# BORING LOG

**BORING NO.:** B-101  
**SHEET:** 2 of 2  
**PROJECT NO.:** 21-0679.2  
**DATE START:** 1/6/2022  
**DATE FINISH:** 1/6/2022

**CLIENT:** Kleinfelder  
**PROJECT:** Chamberlain Street Retaining Wall  
**LOCATION:** Chamberlain Street, Augusta, Maine

Elev. (ft)	Depth (ft)	Casing Pen. (bpf)	SAMPLE INFORMATION					Graphic Log	Sample Description & Classification	H <sub>2</sub> O Depth	Remarks
			Sample No.	Type	Depth (ft)	Pen./ Rec. (in)	Blow Count or RQD				
			10D		25-27	24/24	WOR- WOR- WOH- WOH	ID 13736A w =37.7 % W <sub>L</sub> =42 W <sub>p</sub> =18	25.0	Medium stiff, gray, saturated, Silty CLAY with occasional fine sandy silt partings	
	30		1V		30.6-30.9	4/0	S <sub>v</sub> =0.74/0.37ksf				
	35										
									36.6	ROD PROBE Depth Resistance Interpreted Soil Type 36.6-37.6 45 blows Granular Soil	
									37.6-38.6	47 blows	

Bottom of Exploration at 38.6 feet

BORING / WELL: 21-0679.2.GPJ SWCE TEMPLATE.GDT 2/4/22

Stratification lines represent approximate boundary between soil types, transitions may be gradual. Water level readings have been made at times and under conditions stated. Fluctuations of groundwater may occur due to other factors than those present at the time measurements were made.

**BORING NO.:** **B-101**

## **KEY TO NOTES & SYMBOLS**

### **Test Boring and Test Pit Explorations**

Stratification lines represent the approximate boundary between soil types and the transition may be gradual.

#### **Key to Symbols Used:**

w	-	water content, percent (dry weight basis)
q <sub>u</sub>	-	unconfined compressive strength, kips/sq. ft. - laboratory test
S <sub>v</sub>	-	field vane shear strength, kips/sq. ft.
L <sub>v</sub>	-	lab vane shear strength, kips/sq. ft.
q <sub>p</sub>	-	unconfined compressive strength, kips/sq. ft. – pocket penetrometer test
O	-	organic content, percent (dry weight basis)
W <sub>L</sub>	-	liquid limit - Atterberg test
W <sub>P</sub>	-	plastic limit - Atterberg test
WOH	-	advance by weight of hammer
WOM	-	advance by weight of man
WOR	-	advance by weight of rods
HYD	-	advance by force of hydraulic piston on drill
RQD	-	Rock Quality Designator - an index of the quality of a rock mass.
γ <sub>T</sub>	-	total soil weight
γ <sub>B</sub>	-	buoyant soil weight

#### **Description of Proportions:**

Trace:	0 to 5%
Some:	5 to 12%
“Y”	12 to 35%
And	35+%
With	Undifferentiated

#### **Description of Stratified Soils**

Parting:	0 to 1/16” thickness
Seam:	1/16” to 1/2” thickness
Layer:	½” to 12” thickness
Varved:	Alternating seams or layers
Occasional:	one or less per foot of thickness
Frequent:	more than one per foot of thickness

**REFUSAL: Test Boring Explorations** - Refusal depth indicates that depth at which, in the drill foreman's opinion, sufficient resistance to the advance of the casing, auger, probe rod or sampler was encountered to render further advance impossible or impracticable by the procedures and equipment being used.

**REFUSAL: Test Pit Explorations** - Refusal depth indicates that depth at which sufficient resistance to the advance of the backhoe bucket was encountered to render further advance impossible or impracticable by the procedures and equipment being used.

Although refusal may indicate the encountering of the bedrock surface, it may indicate the striking of large cobbles, boulders, very dense or cemented soil, or other buried natural or man-made objects or it may indicate the encountering of a harder zone after penetrating a considerable depth through a weathered or disintegrated zone of the bedrock.

## **APPENDIX D**

### **Laboratory Test Results**



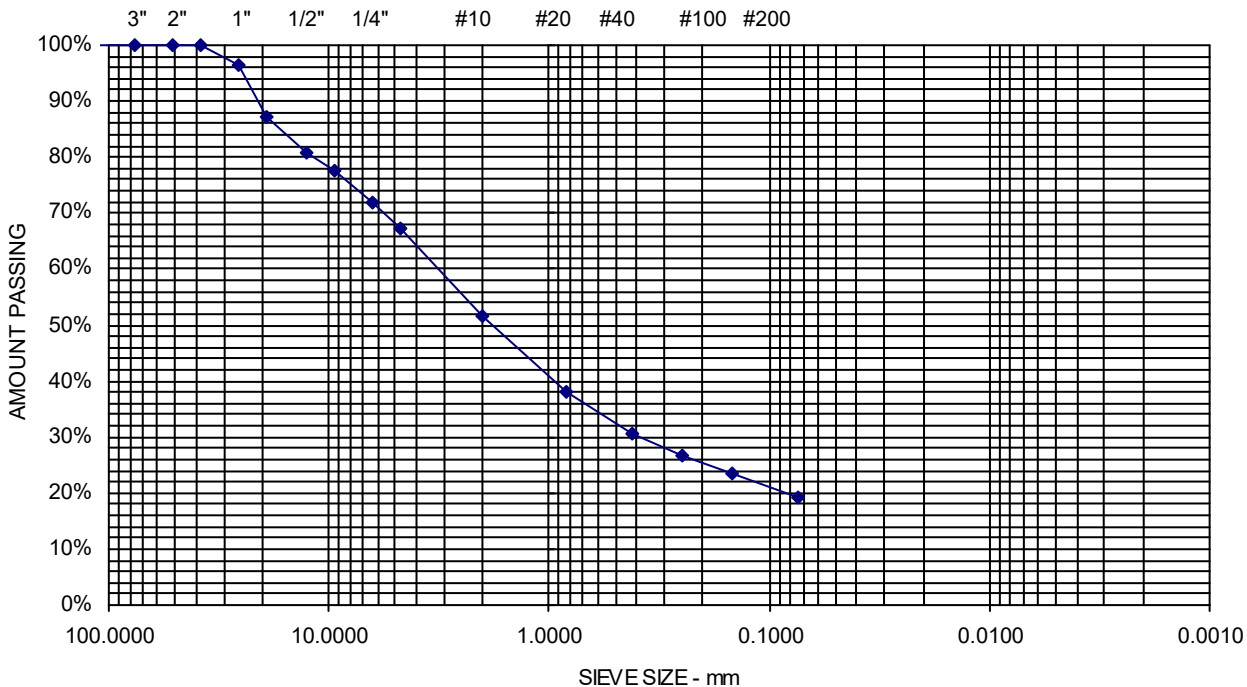
# Report of Gradation

ASTM C-117 & C-136

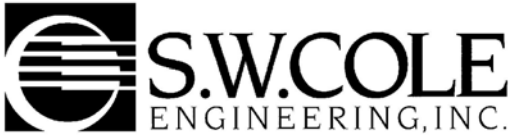
Project Name AUGUSTA ME - PROPOSED RETAINING WALL  
RECONSTRUCTION/REPLACEMENT - GEOTECHNICAL  
Client KLEINFELDER, INC.  
Exploration **B-101**  
Material Source **B-101, 3D, 5-7 FT**

Project Number 21-0679.2  
Lab ID 13733A  
Date Received 1/14/2022  
Date Completed 1/19/2022  
Tested By ERNEST FORGIONE JR

<u>STANDARD DESIGNATION (mm/μm)</u>	<u>SIEVE SIZE</u>	<u>AMOUNT PASSING (%)</u>	
150 mm	6"	100	
100 mm	4"	100	
75 mm	3"	100	
50 mm	2"	100	
38.1 mm	1-1/2"	100	
25.0 mm	1"	96	
19.0 mm	3/4"	87	
12.5 mm	1/2"	81	
9.5 mm	3/8"	78	
6.3 mm	1/4"	72	
4.75 mm	No. 4	67	32.8% Gravel
2.00 mm	No. 10	52	
850 μm	No. 20	38	
425 μm	No. 40	31	48.1% Sand
250 μm	No. 60	27	
150 μm	No. 100	24	
75 μm	No. 200	19.1	19.1% Fines



Comments: As Delivered MC: 4.8%



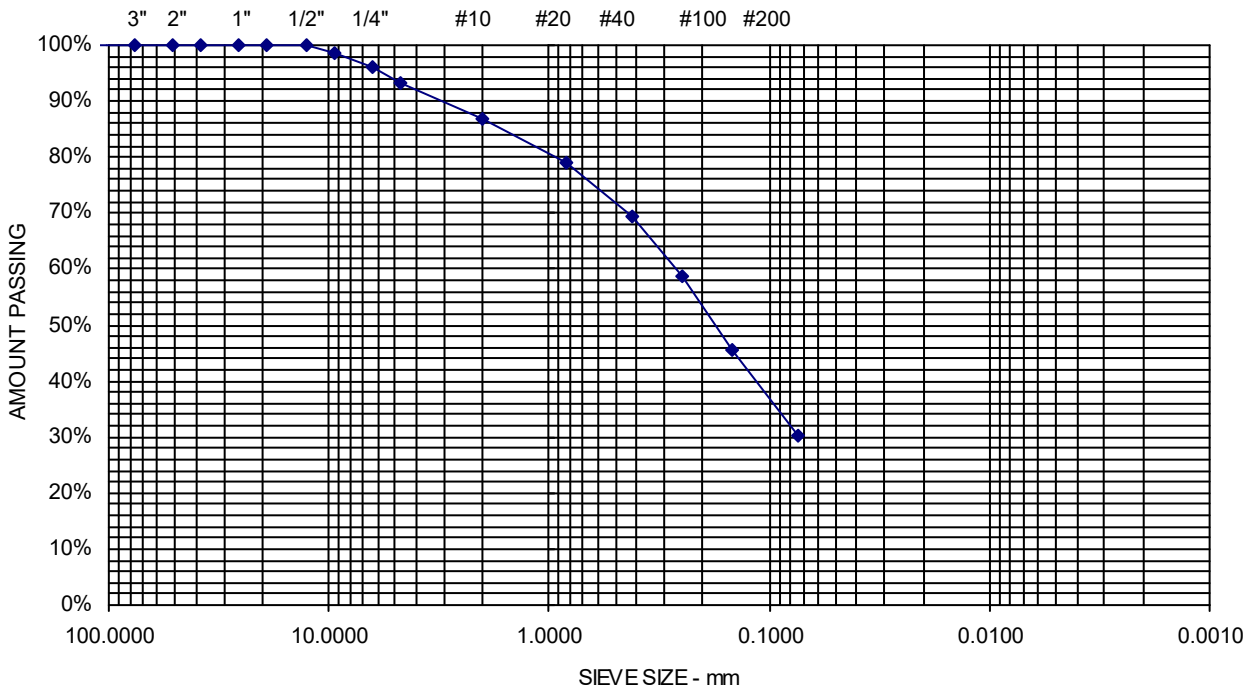
# Report of Gradation

ASTM C-117 & C-136

Project Name AUGUSTA ME - PROPOSED RETAINING WALL  
 RECONSTRUCTION/REPLACEMENT - GEOTECHNICAL  
 Client KLEINFELDER, INC.  
 Exploration **B-101**  
 Material Source **B-101, 7D, 14-16 FT**

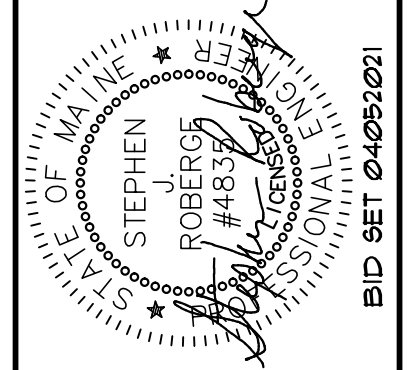
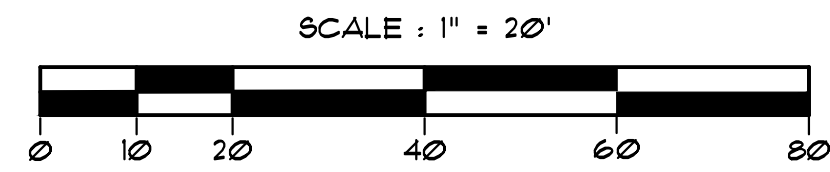
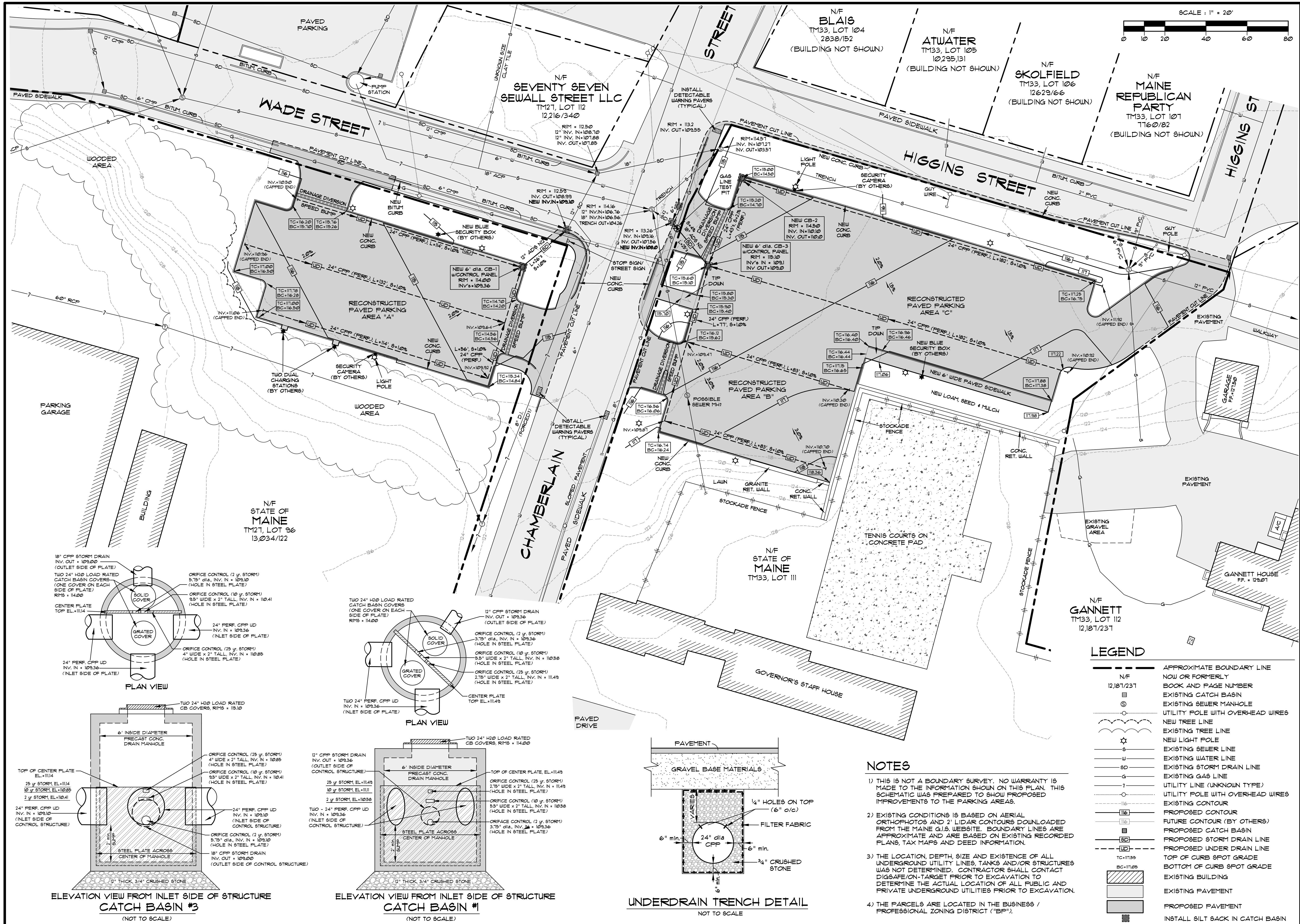
Project Number 21-0679.2  
 Lab ID 13734A  
 Date Received 1/14/2022  
 Date Completed 1/19/2022  
 Tested By BRANDON CHAPUT

<u>STANDARD DESIGNATION (mm/μm)</u>	<u>SIEVE SIZE</u>	<u>AMOUNT PASSING (%)</u>	
150 mm	6"	100	
100 mm	4"	100	
75 mm	3"	100	
50 mm	2"	100	
38.1 mm	1-1/2"	100	
25.0 mm	1"	100	
19.0 mm	3/4"	100	
12.5 mm	1/2"	100	
9.5 mm	3/8"	98	
6.3 mm	1/4"	96	
4.75 mm	No. 4	93	6.7% Gravel
2.00 mm	No. 10	87	
850 μm	No. 20	79	
425 μm	No. 40	69	63.2% Sand
250 μm	No. 60	59	
150 μm	No. 100	45	
75 μm	No. 200	30.1	30.1% Fines



Comments: As Delivered MC: 9.9%





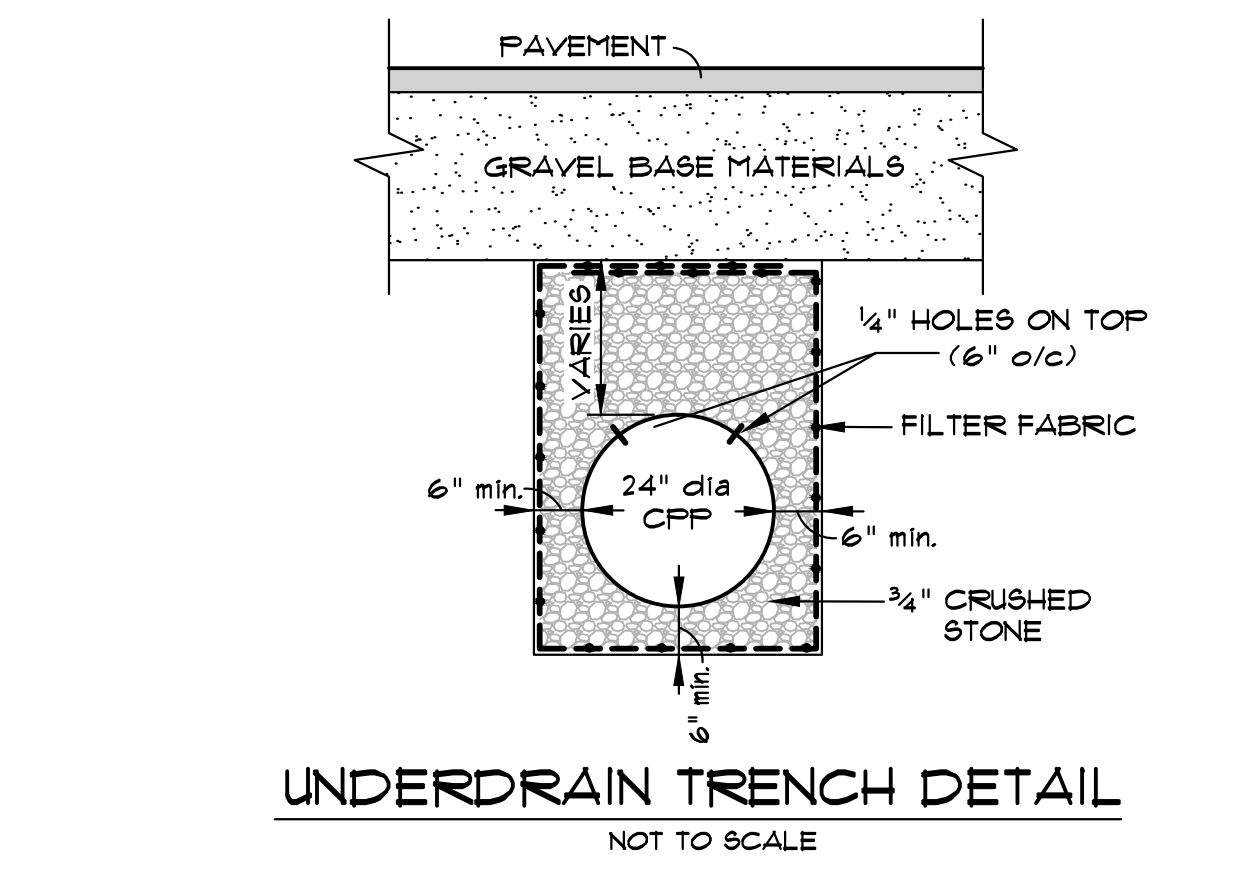
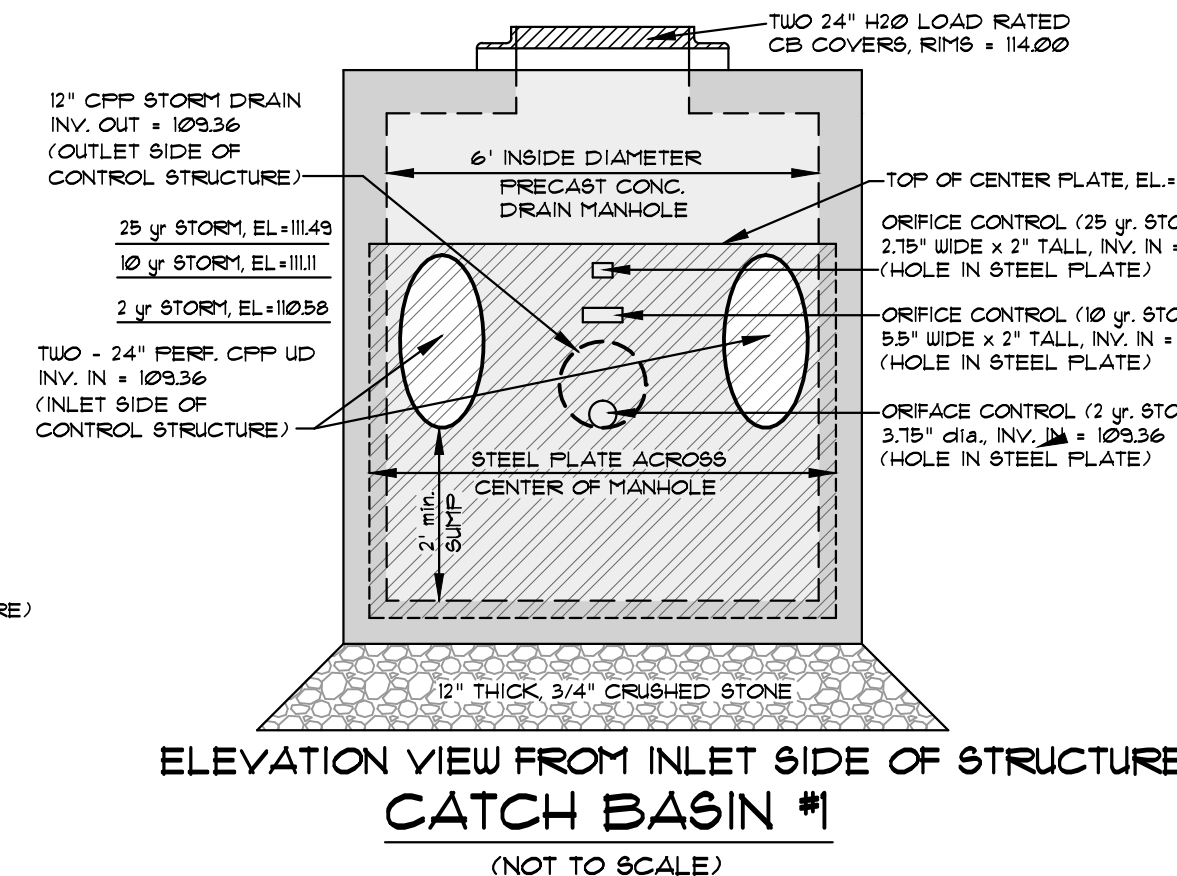
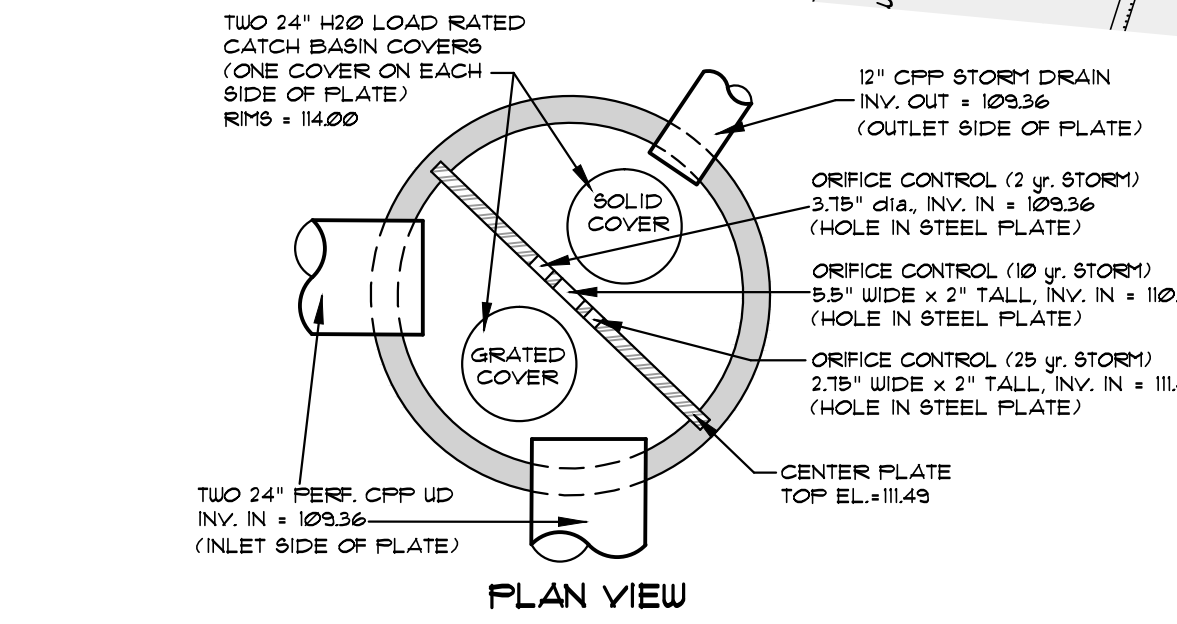
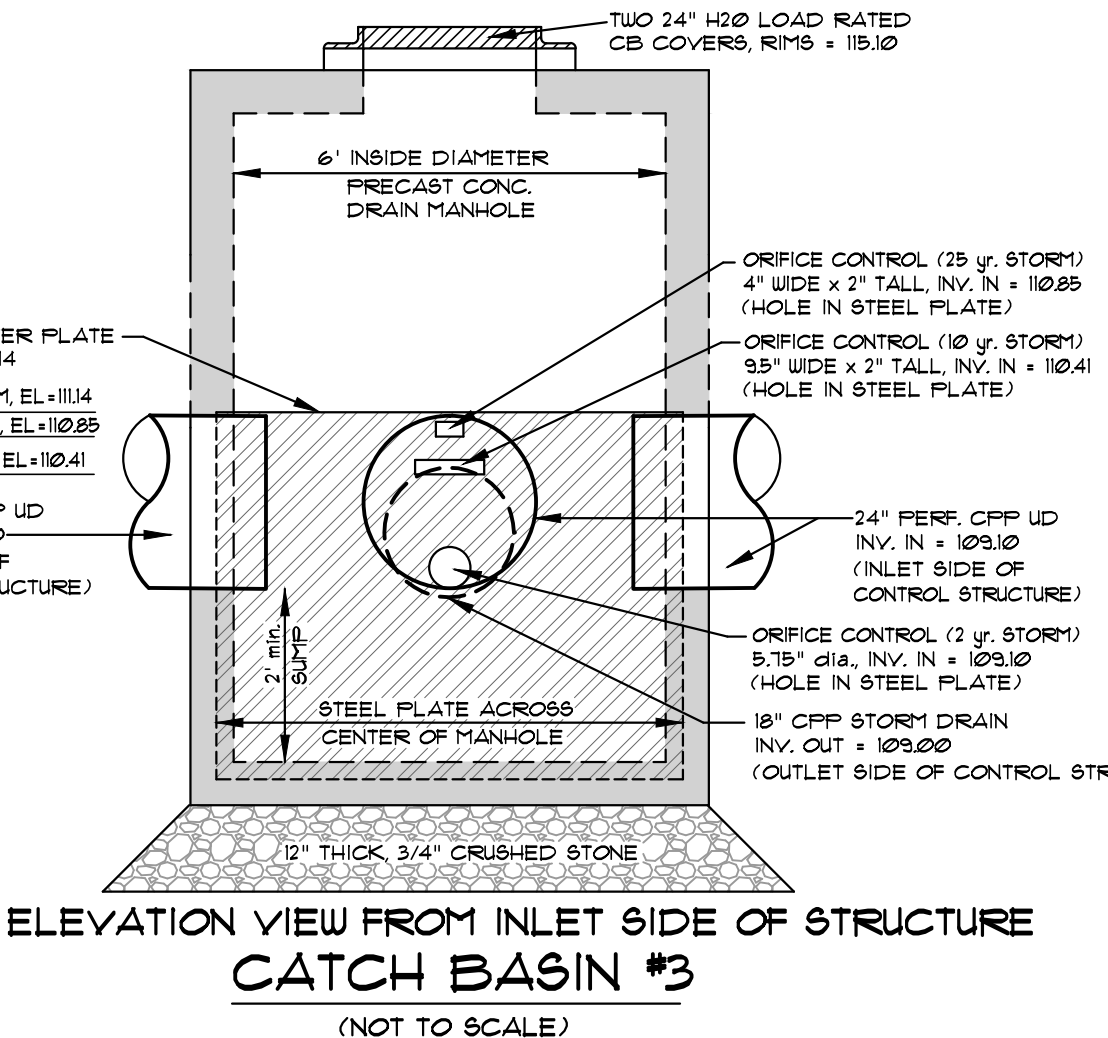
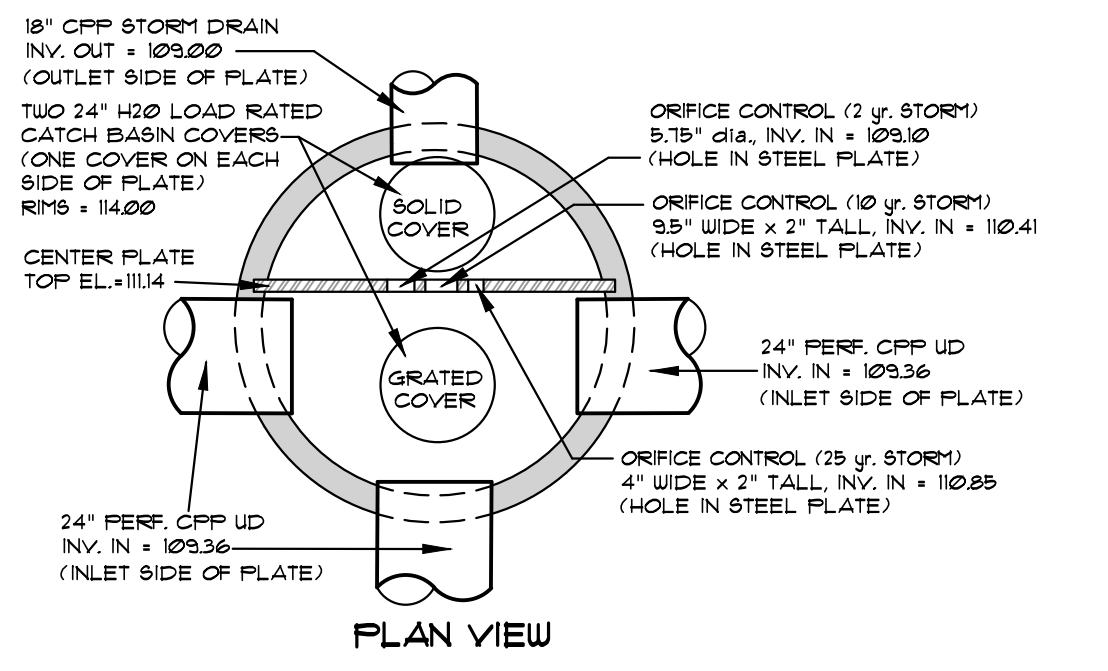
DATE	BY	DESCRIPTION
MAR 19, 2021	SJR	ADD OUTLET CONTROL STRUCTURES
MAR 2, 2021	SJR	REMOVE ABUTTER CONSTRUCTION
MAR 2, 2021	SJR	CHANGES PER CITY REVIEW
FEB 1, 2020	SJR	ADD RIM & INVERT ELEVATIONS
JAN 19, 2020	SJR	RELOCATE LIGHT POLES
JAN 11, 2020	SJR	REMOVE ISLAND, RECONFIGURE
DEC 28, 2019	SJR	ADD GANNETT HOUSE PROPOSED CONDITIONS
	REV:	DATE: CHANGES:

**SJR ENGINEERING, INC.**  
 16 THURSTON DRIVE  
 MONMOUTH, MAINE 04259  
 (207) 242-6248 tel  
 steve@sjreng.com

**TOPOGRAPHIC & UTILITY PLAN**  
**CHAMBERLAIN STREET**  
**PARKING LOT IMPROVEMENTS**  
 PREPARED FOR  
**STATE OF MAINE**  
 BUREAU OF REAL ESTATE MANAGEMENT

DATE	PROJECT
DEC. 2019	2019-99
DRAWN BY	SCALE
SJR	1" = 20'

**SHEET 3**

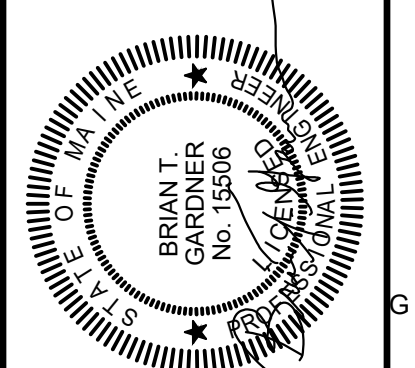
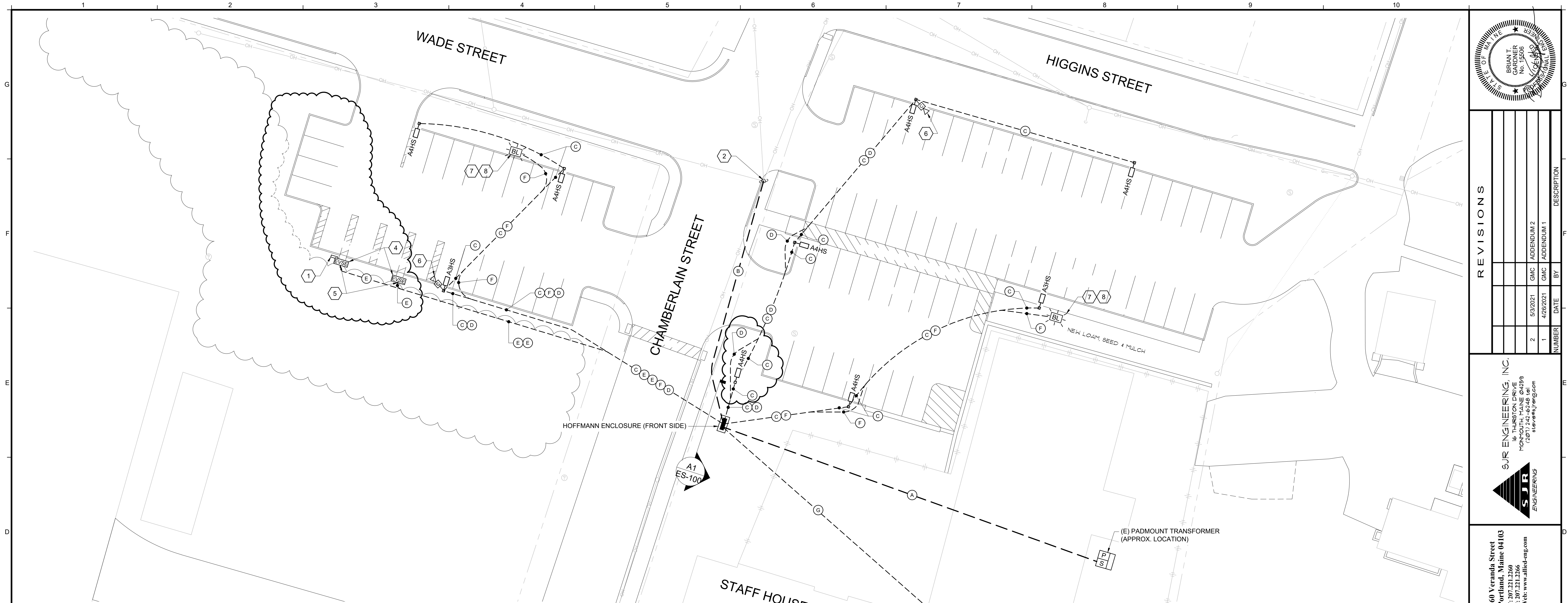


- NOTES**
- THIS IS NOT A BOUNDARY SURVEY. NO WARRANTY IS MADE TO THE INFORMATION SHOWN ON THIS PLAN. THIS SCHEMATIC WAS PREPARED TO SHOW PROPOSED IMPROVEMENTS TO THE PARKING AREAS.
  - EXISTING CONDITIONS IS BASED ON AERIAL ORTHOPHOTOS AND 2" LIDAR CONTOURS DOWNLOADED FROM THE MAINE G.I.S. WEBSITE. BOUNDARY LINES ARE APPROXIMATE AND ARE BASED ON EXISTING RECORDED PLANS, TAX MAPS AND DEED INFORMATION.
  - THE LOCATION, DEPTH, SIZE AND EXISTENCE OF ALL UNDERGROUND UTILITY LINES, TANKS AND/OR STRUCTURES WAS NOT DETERMINED. CONTRACTOR SHALL CONTACT DIGSAFE/ON-TARGET PRIOR TO EXCAVATION TO DETERMINE THE ACTUAL LOCATION OF ALL PUBLIC AND PRIVATE UNDERGROUND UTILITIES PRIOR TO EXCAVATION.
  - THE PARCELS ARE LOCATED IN THE BUSINESS / PROFESSIONAL ZONING DISTRICT ("BP").

**LEGEND**

---	APPROXIMATE BOUNDARY LINE
N/F	NOW OR FORMERLY
12/18/231	BOOK AND PAGE NUMBER
⊕	EXISTING CATCH BASIN
⊙	EXISTING SEWER MANHOLE
○	UTILITY POLE WITH OVERHEAD WIRES
---	NEW TREE LINE
---	EXISTING TREE LINE
☆	NEW LIGHT POLE
---	EXISTING SEWER LINE
---	EXISTING WATER LINE
---	EXISTING STORM DRAIN LINE
---	EXISTING GAS LINE
---	UTILITY LINE (UNKNOWN TYPE)
---	UTILITY POLE WITH OVERHEAD WIRES
---	EXISTING CONTOUR
---	PROPOSED CONTOUR
---	FUTURE CONTOUR (BY OTHERS)
---	PROPOSED CATCH BASIN
---	PROPOSED STORM DRAIN LINE
---	PROPOSED UNDER DRAIN LINE
---	TOP OF CURB SPOT GRADE
---	BOTTOM OF CURB SPOT GRADE
---	EXISTING BUILDING
---	EXISTING PAVEMENT
---	PROPOSED PAVEMENT
---	INSTALL SILT SACK IN CATCH BASIN



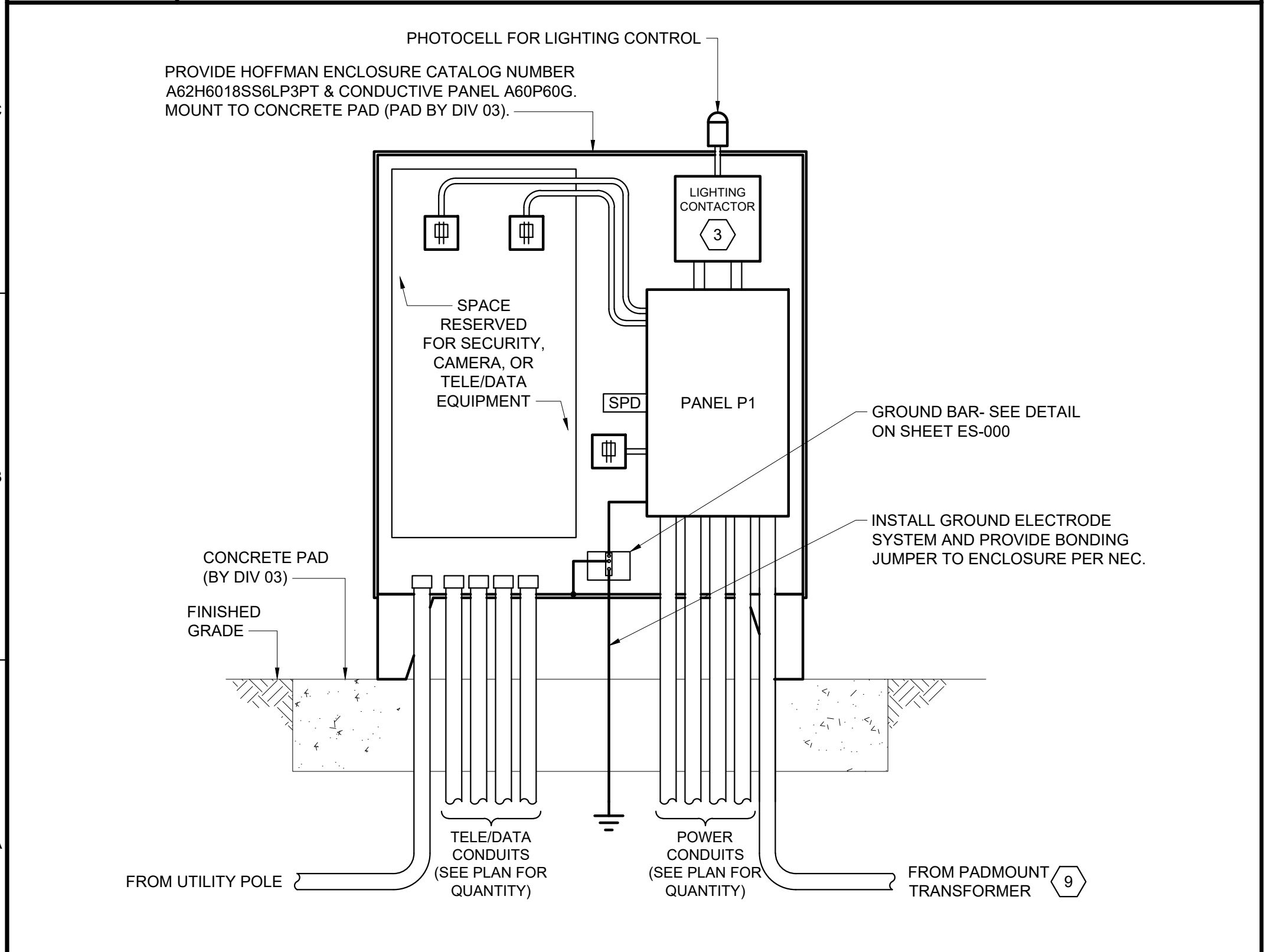


REVISIONS			
NUMBER	DATE	BY	DESCRIPTION
2	5/3/2021	GMC	ADDENDUM 2
1	4/26/2021	GMC	ADDENDUM 1

**SJR ENGINEERING, INC.**  
 160 YERANDA STREET  
 PORTLAND, MAINE 04103  
 T: 207.221.1260  
 F: 207.221.1266  
 WWW.ALIED-ENG.COM

**Allied Engineering**  
 Structural, Mechanical, Electrical Commissioning  
 160 Yeranda Street  
 Portland, Maine 04103  
 T: 207.221.1260  
 F: 207.221.1266  
 WWW.ALIED-ENG.COM

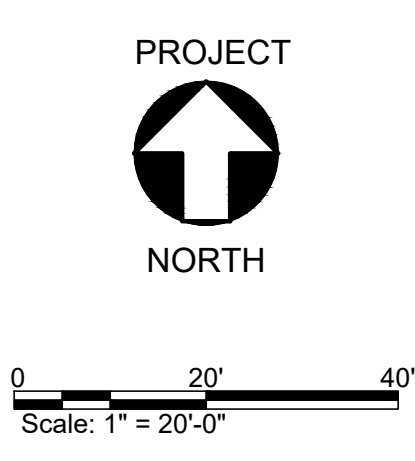
**D1 ELECTRICAL SITE PLAN**  
 SCALE: 1"=20'



**A1 SERVICE BOX DIAGRAM/ELEVATION**  
 NOT TO SCALE

**STAFF HOUSE GARAGE**

- (A) SERVICE ENTRANCE (PAD TO PANEL)  
- (1) 3" C WITH (4)#4/0 + (1)#4G
- (B) TELECOMMUNICATIONS  
- (2) 2" C EMPTY WITH PULLSTRING
- (C) SITE LIGHTING  
- (1) 1" C WITH (2) #10, (1) #10G U.N.O.
- (D) SECURITY CAMERA  
- (2) 1" C EMPTY WITH PULLSTRING
- (E) CHARGING STATIONS POWER & DATA  
- (1) 1" C WITH (4)#6 + (1)#10G FOR POWER  
- (2) 1" C EMPTY EACH WITH PULLSTRING FOR DATA (IN CASE CELLULAR CONNECTION METHOD IS NOT UTILIZED AND HARD-WIRED CONNECTION IS REQUIRED)
- (F) EMERGENCY "BLUE LIGHT" STATIONS  
- (1) 1 1/2" C WITH (2)#12 + (1)#12G FOR POWER  
- (1) 1 1/2" C EMPTY WITH PULLSTRING FOR DATA
- (G) TELECOMMUNICATIONS  
- (1) 1 1/2" C EMPTY WITH PULLSTRING



- 1 EXISTING WEATHERPROOF (WP) SINGLE PHASE 208V, 40A PANEL WITH WEATHERPROOF RECEPTACLES FOR BLOCK HEATERS LOCATED IN APPROXIMATELY THIS AREA, TO BE DISCONNECTED AND REMOVED BY THE OWNER PRIOR TO PARKING LOT RECONSTRUCTION.
- 2 EXISTING UNUSED CUSTOMER-OWNED 75KVA PADMOUNTED TRANSFORMER SHALL BE UTILIZED FOR NEW PARKING LOT POWER PANEL SERVICE. COORDINATE EXACT LOCATION IN FIELD.
- 3 PROVIDE (8) CHANNEL LIGHTING CONTACTOR PANEL WITH HAND/OFF/AUTO SELECTOR SWITCH ON ENCLOSURE COVER (FOR BYPASS OF PHOTOCELL).
- 4 LEVEL 2 NETWORKED ELECTRIC VEHICLE CHARGING STATIONS; CHARGEPOINT MODEL NO. CT4000 (2 CHARGING PORTS PER POST). COORDINATE WITH OWNER FOR DIVISION OF SCOPE WITH REGARD TO THE OWNER'S MASTER AGREEMENT CONTRACT WITH CHARGEPOINT. CONTRACTOR SHALL MAKE THE REQUIRED CONDUIT AND POWER PROVISIONS TO FACILITATE THE EQUIPMENT PROVISION AND INSTALLATION BY THE OWNER'S VENDOR.
- 5 CONDUIT STUB-UP AT UNIT SHALL BE 24" MAX AFG.
- 6 POLE MOUNTED CAMERA EQUIPMENT, INSTALLATION, AND WIRING SHALL BE PROVIDED BY OTHERS. COORDINATE WITH OWNER FOR DIVISION OF SCOPE. CONTRACTOR SHALL MAKE THE REQUIRED CONDUIT AND POWER PROVISIONS TO FACILITATE THE EQUIPMENT PROVISION AND INSTALLATION.
- 7 ECS (BLUE LIGHT) EQUIPMENT, INSTALLATION, AND WIRING SHALL BE PROVIDED BY OTHERS. COORDINATE WITH OWNER FOR DIVISION OF SCOPE. CONTRACTOR SHALL MAKE THE REQUIRED CONDUIT AND POWER PROVISIONS TO FACILITATE THE EQUIPMENT PROVISION AND INSTALLATION.
- 8 CONDUIT STUB-UP AT UNIT SHALL BE 12" MAX ABOVE CONCRETE BASE.
- 9 NEW SECONDARY FROM EXISTING UNUSED CUSTOMER-OWNED PADMOUNTED TRANSFORMER LOCATED APPROXIMATELY 150 FEET FROM FENCE AT THE SOUTHWEST CORNER. TRANSFORMER IS A WESTINGHOUSE, 75 KVA, 3 PHASE, 12.57 KV PRIMARY TO 120/208 VOLT SECONDARY. COORDINATE PRIMARY SHUT DOWN TO TRANSFORMER WITH STATE FACILITY ELECTRICIAN. EXCAVATE UNDER EXISTING TRANSFORMER TO ROUTE NEW SECONDARY CONDUIT FROM SECONDARY WINDOW IN CONCRETE BASE TO PROPOSED CONTROL/POWER CABINET.
- 10 STUB CONDUIT UP INTO THE INSIDE OF THE EXISTING STAFF HOUSE GARAGE (WEST MOST BAY NEAR THE EXISTING SECURITY PANELS). EXTEND CONDUIT TO A MINIMUM OF 36" AFF (GARAGE SLAB). COORDINATE WITH OWNER'S REPRESENTATIVE IN THE FIELD FOR THE EXACT LOCATION OF THE STUB UP.

**A7 CONDUIT KEY**

**A9 KEYNOTES**

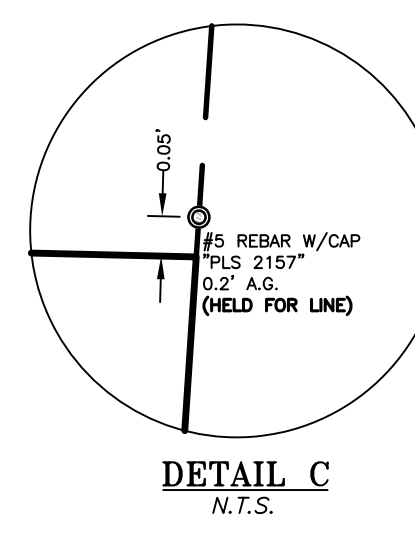
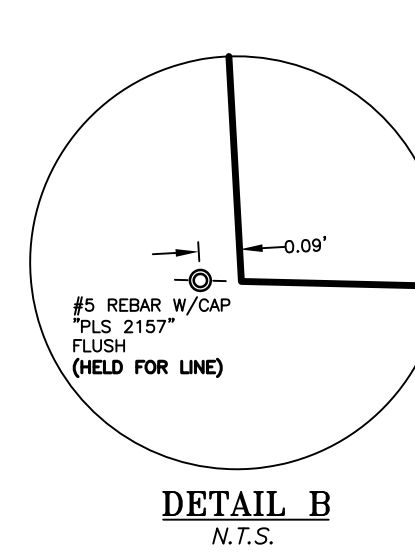
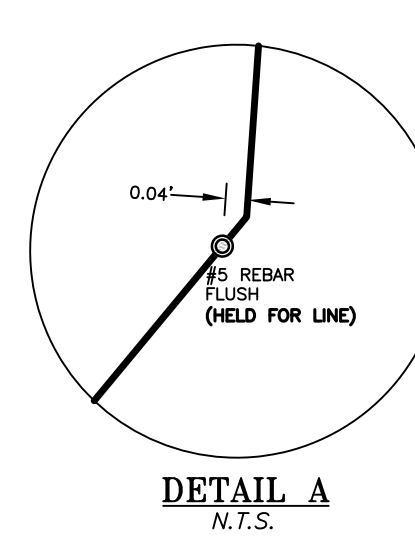
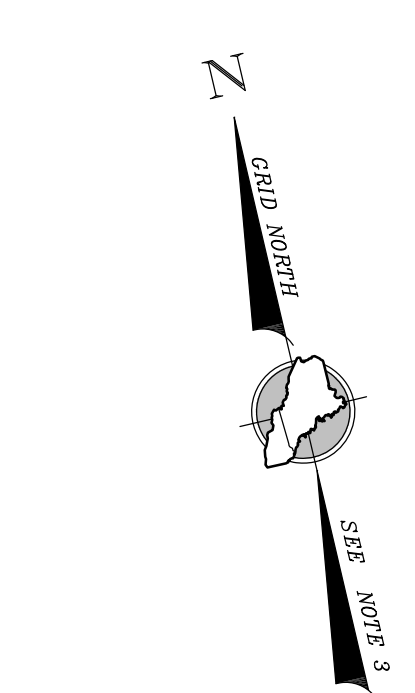
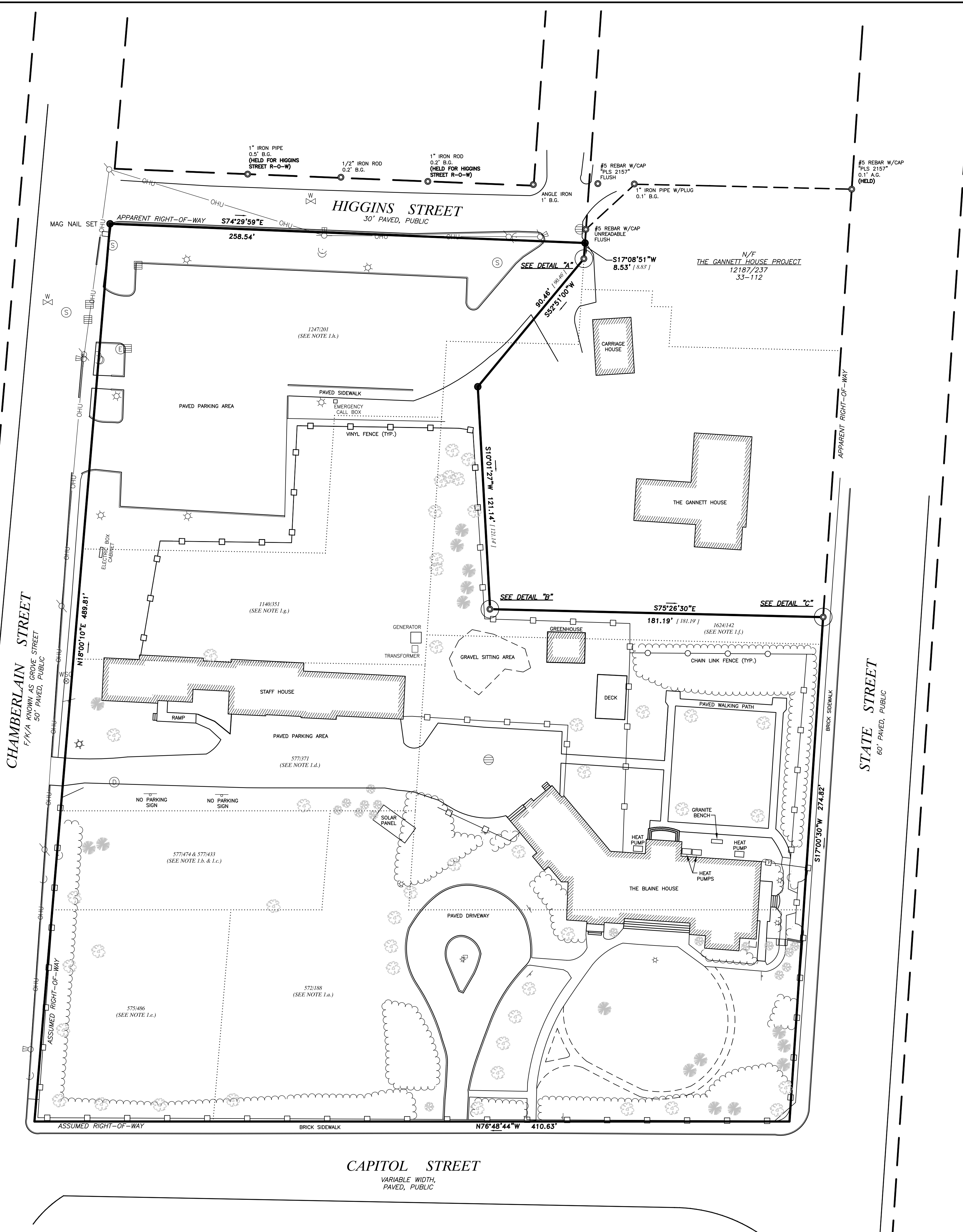
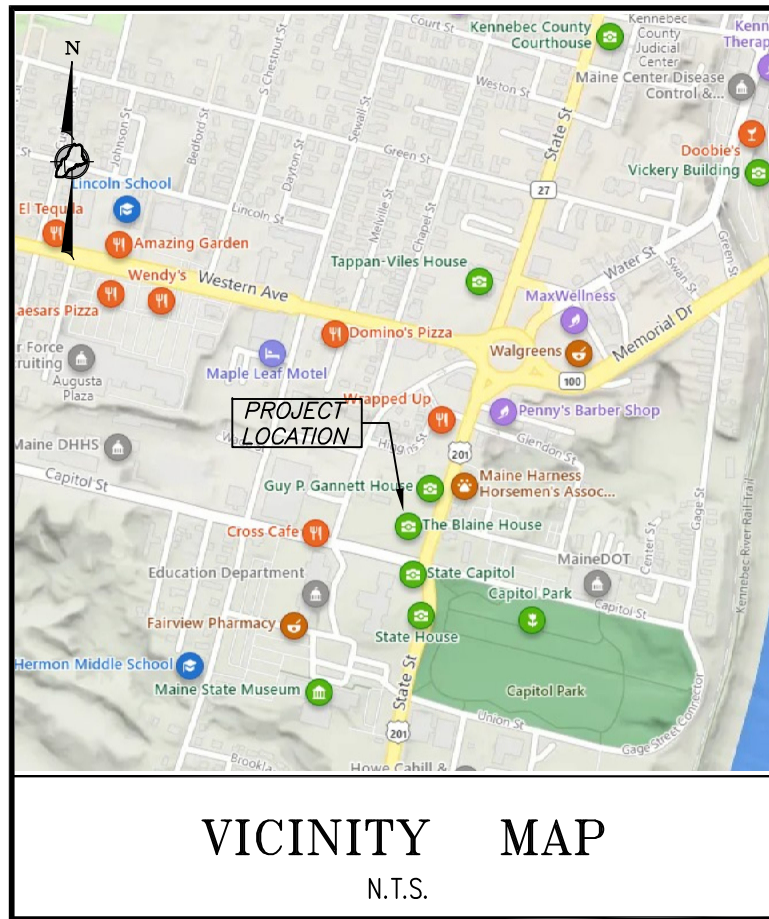
**CHAMBERLAIN STREET  
 PARKING LOT IMPROVEMENTS**  
 PREPARED FOR  
 STATE OF MAINE  
 BUREAU OF REAL ESTATE MANAGEMENT

DATE	PROJECT
2021-04-09	2019-39
DRAWN BY	SCALE
GMC	AS NOTED

N:\Projects\2021\21031 - Chamberlain St. Parking Lot\4e DRAWINGS\21031-ES.dwg  
 May 03, 2021 - 10:46am  
 A1

ISSUED FOR CONSTRUCTION - 04-09-2021





### LEGEND

- #5 REBAR WITH CAP (NCS 2080), OR AS NOTED, SET ON 7/12/22
- FOUND IRON (SIZE & TYPE AS NOTED)
- UTILITY POLE (NUMBER AS NOTED)
- GUY WIRE ANCHOR
- FOUND DECIDUOUS TREE (SIZE AS NOTED)
- FOUND CONIFEROUS TREE (SIZE AS NOTED)
- TREE LINE (APPROXIMATE)
- BOUNDARY LINE
- EASEMENT LINE
- EDGE OF GRAVEL
- EDGE OF PAVEMENT
- RIGHT-OF-WAY LINE
- ABUTTER LINE
- OVERHEAD UTILITY
- FORMER DEED LINES
- NOW OR FORMERLY OWNED BY
- DEED BOOK AND PAGE
- TAX MAP-LOT
- BRACKETS DENOTE RECORD DATA FROM PLAN REFERENCE 4.e.

- ### NOTES
- RECORD OWNER OF THE PARCEL SURVEYED IS STATE OF MAINE AS DESCRIBED IN THE FOLLOWING DEEDS:
    - 572/188 FROM HARRIET BLAINE BEALE, DATED MARCH 10, 1919.
    - 577/474 FROM FRANCES MORSE, DATED JULY 29, 1919.
    - 574/433 FROM FRANCES MORSE, DATED JULY 29, 1910.
    - 577/371 FROM HENRY T. CLARK, DATED JULY 19, 1919.
    - 575/486 FROM ELIZABETH N. WHEELER, DATED JULY 29, 1919.
    - 1624/142 FROM MAINE TEACHERS ASSOCIATION, DATED MARCH 29, 1973.
    - 1140/351 FROM WALTER M. SANBORN AND GRACE J. ROLLINS, EXECUTORS AND TRUSTEES UNDER THE WILL OF BERTHA N. CROCKER, DATED DECEMBER 24, 1958.
    - 1247/201 FROM W.M. WEBSTER, DATED NOVEMBER 27, 1961.

TOTAL LOT AREA IS: 3.6± Ac.
  - THE PARCEL SURVEYED IS IDENTIFIED ON THE CITY OF AUGUSTA TAX ASSESSOR'S MAP 33, PARCELS 111 AND 119.
  - THE BEARINGS SHOWN ON THIS PLAN ARE BASED ON MAINE COORDINATE SYSTEM OF 1983, WEST ZONE, GRID NORTH.
  - PLAN REFERENCES:
    - "BOUNDARY SURVEY OF THE GANNETT HOUSE FOR THE STATE OF MAINE" BY MORIN LAND SURVEYING, DATED APRIL 21, 2014, UNRECORDED.
    - STATE OF MAINE D.O.T. RIGHT-OF-WAY MAP STATE AID HIGHWAY NO.14, AUGUSTA, FEDERAL AID PROJECT NO. M-1150(1), D.O.T. FILE NO. 6-275, DATED JUNE 1986, SHEET 2 OF 4.
    - STATE OF MAINE D.O.T. RIGHT-OF-WAY MAP STATE AID HIGHWAY NO.14, AUGUSTA, FEDERAL AID PROJECT NO. M-1150(1), D.O.T. FILE 6-275, DATED JUNE, 1986, SHEET 3 OF 4.
    - MAINE STATE HIGHWAY COMMISSION RIGHT-OF-WAY MAP STATE HIGHWAY "1", AUGUSTA, S.H.C. FILE NO. 6-154, DATED JUNE 1964, RECORDED IN PLAN BOOK 33, PAGE 35.
    - "PLAN OF LAND OWNED BY AUGUSTA REAL ESTATE ASSOC. BETWEEN STATE AND GROVE STREETS, AUGUSTA, MAINE" BY W.W. HILL, ENGINEER, DATED JANUARY 25, 1939, RECORDED IN PLAN BOOK 12, PAGE 60.
    - STATE OF MAINE D.O.T., MAINE STATE HIGHWAY COMMISSION, AUGUSTA, HIGHWAY OFFICE BUILDING SITE, S.H.C. FILE NO 6-151, DATED JUNE, 1966.
  - THE WIDTH OF STATE STREET IS 60' BASED ON PLAN REFERENCE 4.c., 4.b. AND 4.d. THE LAYOUT OF THE RIGHT-OF-WAY LIMITS IS BASED ON PLAN REFERENCE 4.c., 4.b. AND 4.d. AND MONUMENTATION FOUND.  
THE WIDTH OF CAPITOL STREET VARIES BASED ON PLAN REFERENCE 4.c. THE LAYOUT OF THE RIGHT-OF-WAY LIMITS IS BASED ON PLAN REFERENCE 4.c.  
THE WIDTH OF CHAMBERLAIN STREET IS 50' BASED ON PLAN REFERENCE 4.c. THE LAYOUT OF THE RIGHT-OF-WAY LIMITS IS BASED ON PLAN REFERENCE 4.c.  
THE WIDTH OF HIGGINS STREET IS 30' BASED ON PLAN REFERENCE 4.c. THE LAYOUT OF THE RIGHT-OF-WAY LIMITS IS BASED ON PLAN REFERENCE 4.c. AND MONUMENTATION FOUND.
  - AS OF THE ORIGINAL DATE OF THIS PLAN, THE PARCEL SURVEYED IS LOCATED IN THE BP ZONE. OTHER MUNICIPAL AND STATE OVERLAY ZONES MAY EXIST AND APPLY. BEFORE PROCEEDING ON ANY PROJECT WE RECOMMEND VERIFYING CURRENT ZONE AND ALL APPLICABLE SETBACKS AND RESTRICTIONS WITH THE APPROPRIATE AGENCIES.
  - THE UTILITIES SHOWN ON THIS PLAN WERE FROM FIELD OBSERVATION ONLY. THERE MAY BE OTHER UTILITIES EXISTING THAT ARE NOT SHOWN. CONTACT DIG-SAFE (888)DIG-SAFE PRIOR TO ANY EXCAVATION WORK.

Revision:	By:	Date:	Change:

PROJECT: 42643      DRAWING NAME: 42643.dwg  
 ISSUED: MAY 5, 2022      SCALE: 1"=30'      FB # 430      DRAWN BY: ASF  
 FIELD BY: ASF/TFM/CJB      FIELD DATE: 4/6/2022      CHECKED BY: DMM / TFM

Drawing Name and Location:  
**BOUNDARY SURVEY**  
 192 STATE STREET, AUGUSTA, MAINE

Owner:  
**STATE OF MAINE**  
 STATE HOUSE, AUGUSTA, MAINE

Prepared For:  
**BUREAU OF GENERAL SERVICES**

**Northeast Civil Solutions**  
 INCORPORATED  
 381 PAYNE ROAD, SCARBOROUGH, MAINE 04074

tel 207.883.1000      fax 207.883.1001      e-mail / website info@northeastcivilsolutions.com www.northeastcivilsolutions.com

### STAMP AND SIGNATURE

TROY F. McDONALD  
 MAINE PROFESSIONAL LAND SURVEYOR No. 2080

01/14/2022  
 DATE



**00 41 13  
Contractor Bid Form**

**Chamberlain Street Parking Retaining Wall**

3172

Bid Form submitted by: *email only to email address below*

Bid Administrator:

*Jill Instasi*  
Bureau of General Services  
111 Sewall Street, Cross State Office Building, 4th floor  
77 State House Station  
Augusta, Maine 04333-0077

BGS.Architect@Maine.gov

Bidder:

Signature: \_\_\_\_\_

Printed name and  
title: \_\_\_\_\_

Company name: \_\_\_\_\_

Mailing address: \_\_\_\_\_

City, state, zip code: \_\_\_\_\_

Phone number: \_\_\_\_\_

Email address: \_\_\_\_\_

State of  
incorporation,  
if a corporation: \_\_\_\_\_

List of all partners,  
if a partnership: \_\_\_\_\_

The Bidder agrees, if the Owner offers to award the contract, to provide any and all bonds and certificates of insurance, as well as Schedule of Values, Project Schedule, and List of Subcontractors and Suppliers if required by the Owner, and to sign the designated Construction Contract within twelve calendar days after the date of notification of such acceptance, except if the twelfth day falls on a State of Maine government holiday or other closure day, or a Saturday, or a Sunday, in which case the aforementioned documents must be received before 12:00 noon on the first available business day following the holiday, other closure day, Saturday, or Sunday.

As a guarantee thereof, the Bidder submits, together with this bid, a bid bond or other acceptable instrument as and if required by the Bid Documents.

**00 41 13  
Contractor Bid Form**

1. The Bidder, having carefully examined the *Chamberlain Street Retaining Wall* Project Manual dated March 9, 2023, prepared by Kleinfelder, as well as Specifications, Drawings, and any Addenda, the form of contract, and the premises and conditions relating to the work, proposes to furnish all labor, equipment and materials necessary for and reasonably incidental to the construction and completion of this project for the **Base Bid** amount of:

\$ \_\_\_\_\_ .00

2. Allowances *are not included* on this project.

*No Allowances*

*insert brief name of Allowance*

\$ 0.00

3. Alternate Bids *are not included* on this project.

*No Alternate Bids*

Any dollar amount line below that is left blank by the Bidder shall be read as a bid of **\$0.00**.

*1 insert title of Alternate or "not used"*

\$ \_\_\_\_\_ .00

*2 insert title of Alternate or "not used"*

\$ \_\_\_\_\_ .00

*3 insert title of Alternate or "not used"*

\$ \_\_\_\_\_ .00

*4 insert title of Alternate or "not used"*

\$ \_\_\_\_\_ .00

4. Bid security *is required* on this project.

If noted above as required, or if the Base Bid amount exceeds \$125,000.00, the Bidder shall include with this bid form a satisfactory Bid Bond (section 00 43 13) or a certified or cashier's check for 5% of the bid amount with this completed bid form submitted to the Owner.

5. Filed Sub-bids *are not required* on this project.

If noted above as required, the Bidder shall include with this bid form a list of each Filed Sub-bidder selected by the Bidder on the form provided (section 00 41 13F).



